

Presented by:

Tim Holtermann – Canam Buildings Jeff Martin – Vulcraft-Verco



# **Polling Question**

- New requirement to earn PDH credits
- Two questions will be asked during the duration of today's presentation
- The question will appear within the polling section of your GoToWebinar Control Panel to respond



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#### Learning Objectives

- Understand the requirements for joist and Joist Girder anchorage to supports.
- Review bridging connections and connections that are part of lateral load resisting systems.
- Understand the options for hanging loads from joists, either from structural or trade elements.
- Identify the deck to support connection types and details.
- Summarize deck connection patterns and applications.
- Review deck hanging load connections.



#### Simple Joist Connection Topics

- ☐ Joist to Support
- ☐ Joist Girder to Support
- ☐ Bridging Connections
- ☐ Lateral Load Connections
- ☐ Structural Elements Connected to Joists
- ☐ Trade Elements Connected to Joists

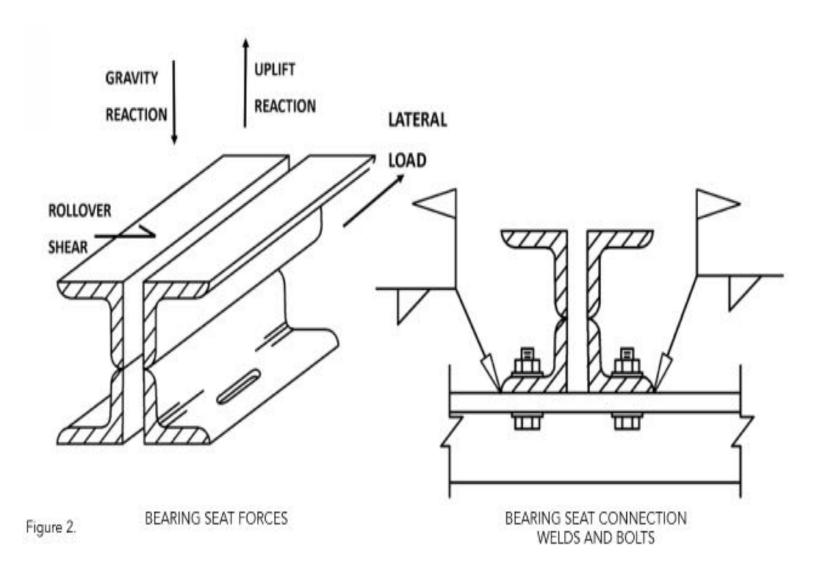


Structurally, a welded joist seat to support connection is adequate, but OSHA requires bolts in certain cases — tie joists and spans over 40 feet on steel supports.

Bolts in slotted holes may not provide adequate lateral support to the compression chord/flange of the supporting member.

Hence, bolted and welded joist end anchorage is common.





7



SJI requirements – weld lengths have increased to engage seat length for uplift resistance (prying action).

JOIST SECTION NUMBER <sup>1</sup>	MINIMUM FILLET WELD	MINIMUM BEARING SEAT BOLTS FOR ERECTION				
K1-12	2- 1/8" x 2 1/2" (3 x 64 mm)	2- 1/2" (13 mm) A307				
LH02-06	2-3/16" x 2 1/2" (5 x 64 mm)	2- 1/2 (13 11111) A307				
LH07-17, DLH10-17, JG	2- 1/4" x 2 1/2" (6 x 64 mm)	2-3/4" (19 mm) A307				
DLH18-25, JG <sup>2</sup>	2- 1/4" x 4" (6 x 102 mm)	2- 3/4" (19 mm) A325				

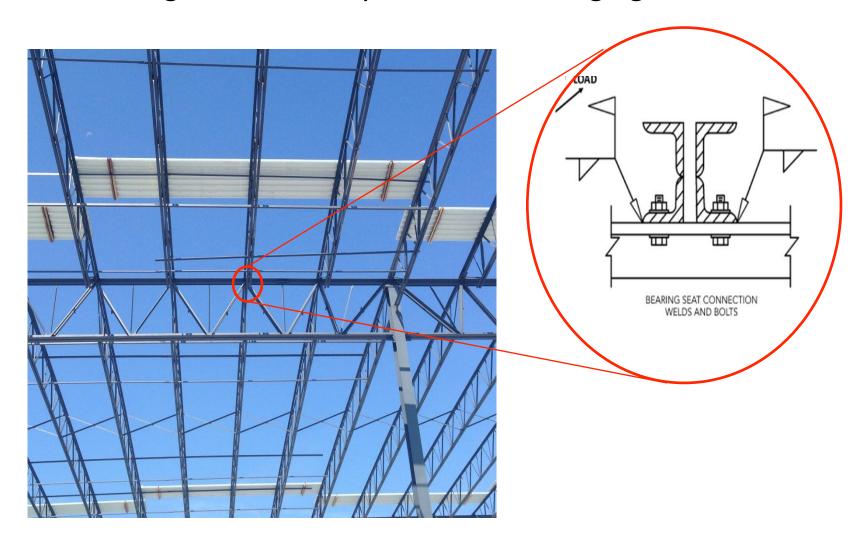
<sup>(1)</sup>Last digit(s) of joist designation shown in load table.

Suggestion: While end anchorage is reduced for LH02-06, it may be easiest to combine them with LH07-17.

<sup>(2)</sup> Joist Girders with a self weight greater than 50 plf (0.73 kN/m).



Joist bearing seat bolts only need to be "snug tight".





# Joist to Joist Girder Support

Typical joist girder web member configurations limit access for the use of power tools for bolt tensioning.





#### OSHA rule for bolted seats

- (8) Field-bolted joists.
  - (i) Except for steel joists that have been pre assembled into panels, connections of individual steel joists to steel structures in bays of 40 feet (12.2 m) or more shall be fabricated to allow for field bolting during erection.
  - These connections shall be field-bolted unless constructability does not allow.

So, while a bolted connection must be detailed, it may or may not be actually used.



Where a joist seat has been detailed for a bolted connection, and for any reason the bolt is not utilized, the empty slot in the bearing seat leg severely diminishes uplift capacity. In such a condition, the weld should be applied within the empty slot.

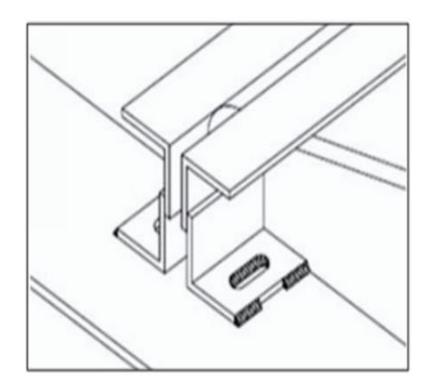


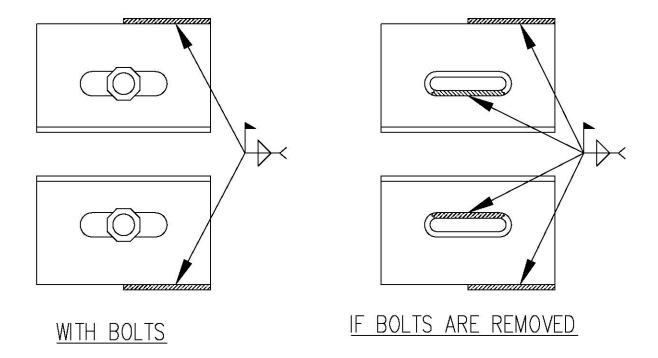
Figure 2.10-1



Typical drawing note and detail

#### **ERECTOR NOTE:**

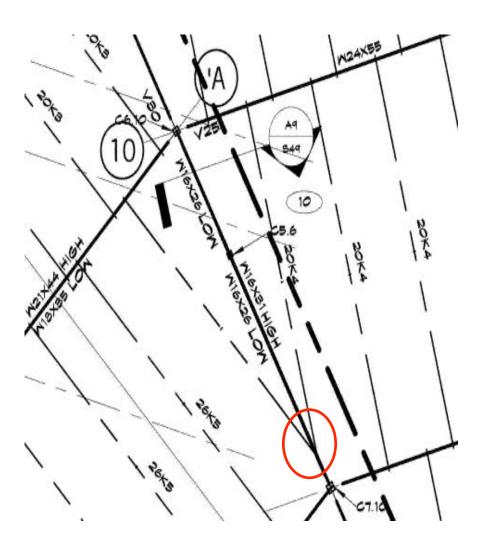
WHERE JOIST ARE FABRICATED TO ALLOW FOR FIELD BOLTING TO THE SUPPORTING STRUCTURE, THE BOLTED CONNECTIONS ARE FOR INITIAL ATTACHMENT ONLY, UNLESS SPECIFICALLY DIRECTED BY THE ENGINEER OF RECORD. SNUG—TIGHTENED BOLTS SHALL REMAIN IN THE BEARING SEAT SLOTS AFTER FINAL CONNECTION IS MADE VIA WELDING PER THE CONTRACT STRUCTURAL DOCUMENTS. IF A BOLTED CONNECTION IS NOT USED, OR THE BOLTS ARE REMOVED AFTER ERECTION, JOIST SEATS MUST BE WELDED ALONG THE INSIDE EDGE OF SEAT SLOTS.





# Joist to Support – Skewed Bearing

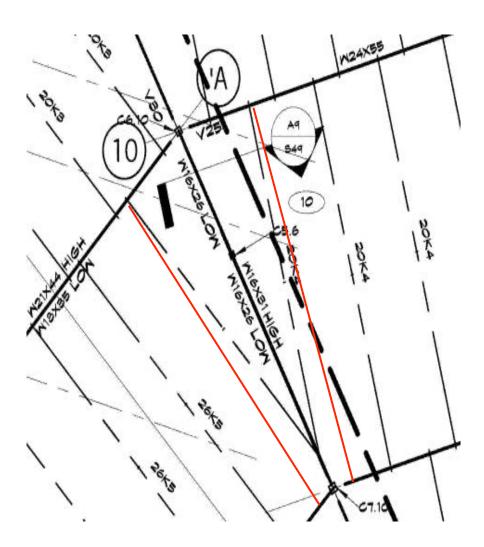
Where the angle of intersection between a joist and supporting beam would be less than 15 degrees, use alternates to avoid an eccentric connection to the beam.





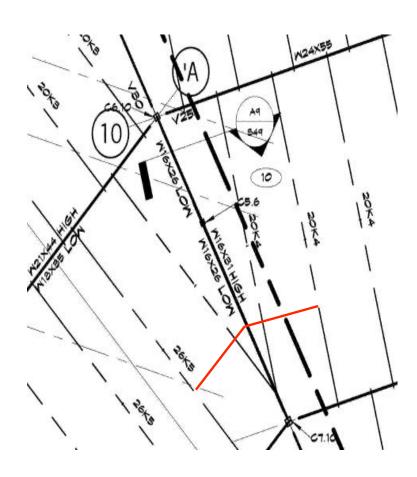
# Joist to Support – Skewed Bearing

Consider skewing the joist. (The deck is not likely to lap on this joist anyway.)





# Joist to Support – Skewed Bearing



Or a header can be used to support the joist end, between the skewed beam and the next joist.



## Simple Joist Connection Topics

- Joist to Support
- ☐ Joist Girder to Support
- ☐ Bridging Connections
- ☐ Lateral Load Connections
- ☐ Structural Elements Connected to Joists
- ☐ Trade Elements Connected to Joists



#### Joist Girder to Support

SJI requirements for Joist Girder end anchorage vary based upon the Joist Girder self weight.

JOIST SECTION NUMBER <sup>1</sup>	MINIMUM FILLET WELD	MINIMUM BEARING SEAT BOLTS FOR ERECTION
K1-12	2- 1/8" x 2 1/2" (3 x 64 mm)	2- 1/2" (13 mm) A307
LH02-06	2-3/16" x 2 1/2" (5 x 64 mm)	2- 1/2 (13 11111) A307
LH07-17, DLH10-17, JG	2- 1/4" x 2 1/2" (6 x 64 mm)	2-3/4" (19 mm) A307
DLH18-25, JG <sup>2</sup>	2- 1/4" x 4" (6 x 102 mm)	2- 3/4" (19 mm) A325

<sup>(1)</sup>Last digit(s) of joist designation shown in load table.

Suggestions: Use A325 bolts for all Joist Girder end anchorage. Use the reduced weld length only if ALL Joist Girders on the project are less than 50 plf.

<sup>(2)</sup> Joist Girders with a self weight greater than 50 plf (0.73 kN/m).



# Joist Girder to Support

Use the Joist Girder
Weight
Tables to find joist girders that weigh over 50 plf.

			JOIST GIRDER WEIGHT - LOAD ON EACH P.										
GIRDER	JOIST	GROER											
SPAN	SPACES	DEPTH	6	В	10	12	14	16	18	20	24	28	33
(0)	E	Dni	9	12	15	18	21	24	27	30	36	42	4)
		36	26	31	37	45	52	59	66	71	87	111	11
	33300000	40	23	29	35	41	46	52	59	68	77	92	11
	5N@	44	22	27	32	37	44	48	54	61	69	80	9
	9.60	48	21	25	30	36	40	48	48	55	69	78	9
	0.000	52	21	25	29	33	39	42	50	54	62	71	83
		56	21	24	29	33	38	40	46	50	59	71	79
- 1		36	28	35	42	51	62	70	78	83	100	122	13
		40	25	33	39	47	56	64	71	79	93	112	12
	6N@	44	24	31	36	45	50	57	65	73	81	102	11
	8.00	48	23	30	35	40	48	52	59	67	78	95	10
	0.0000000	52	23	27	32	38	46	51	59	60	75	83	97
		56	22	27	31	37	42	48	54	61	69	80	9
		36	36	45	56	64	78	91	100	122	134	153	16
		40	33	42	51	59	70	80	92	101	124	148	15
48 8N@	8N@	44	32	39	49	55	65	74	82	95	114	127	15
40	6.00	48	30	37	47	53	60	68	76	84	105	129	13



# Joist Girder to Support

The Joist Girder uplift reaction could exceed the capacity of the SJI minimum bolts.

Two bolts, ¾" A325, have a limit of about 24 kips with prying action considered, or 39 kips max tension capacity with a stiffened seat.





## Simple Joist Connection Topics

- ✓ Joist to Support
- ☑ Joist Girder to Support
- ☐ Bridging Connections
- ☐ Lateral Load Connections
- ☐ Structural Elements Connected to Joists
- ☐ Trade Elements Connected to Joists



#### **Bridging Connections - Welded**

**TABLE 5.5-2** 

BRIDGING NOMINAL HORIZONTAL UNFACTORED COMPRESSSIVE FORCE REQUIRED HORIZONTAL DIAGONAL BRIDGING BRIDGING BRIDGING CONNECTION JOIST SECTION P<sub>br</sub> (n=2) P<sub>br</sub> (n=8) WELD<sup>2</sup> NUMBER1 Lbs. (N) ln. Lbs. K1-8 340 (1512)(378)1/8" x 1" K9-10, LH02-03 450 (2002)113 (503)(3mm x 25mm) 560 140 (623)K11-12, LH04-05 (2491)LH06-08 750 (3336)188 (836)LH09 850 (3781)213 (945)1/8" x 1" LH/DLH10 900 (4003)225 (1001)(3mm x 25mm) LH/DLH11 950 (4226)(1056)LH/DLH12 1100 (4893)275 (1223)LH/DLH13 1200 (5338)300 (1334)LH/DLH14 1300 (5783)325 (1446)1/8" x 1 1/2 " LH/DLH15 1450 (6450)363 (1612)(3mm x 38mm) LH/DLH16-17 1850 (8229)1/8" x 1 1/2 " 463 (2057)(3mm x 38mm) (10453)DLH18-20 2350 (2602)1/8" x 2" 1/8" x 2" DLH21-22 (14012)3150 (3514)(3mm x 51mm) (3mm x 51mm) DLH23-24 4130 (18371)1/8" x 3" 1035 (4604)1/8" x 3" (3mm x 76mm) (21218)DLH25 4770 1195 (5316)(3mm x 76mm) (1) Last digit(s) of joist designation shown in Load Table. (2) Or other connection type designed for the required force.

Bridging connection weld requirements are now provided in the SJI SPEC



## **Bridging Connections - Bolted**

# Bridging connection bolt requirements are tabulated in the SJI COSP

**TABLE 2.7-5** 

BOLT SIZES WHICH MEET BOLTED BRIDGING CONNECTION REQUIREMENTS					
JOIST SERIES	SECTION NUMBER*	BOLT DIAMETER			
K	ALL	3/8" (10 mm) A307			
LH/DLH	2 – 12	3/8" (10 mm) A307			
LH/DLH	13 – 17	1/2" (13 mm) A307			
DLH	18 – 20	5/8" (16 mm) A307			
DLH	21 – 22	5/8" (16 mm) A325			
DLH	23 – 25	3/4" (19 mm) A325			

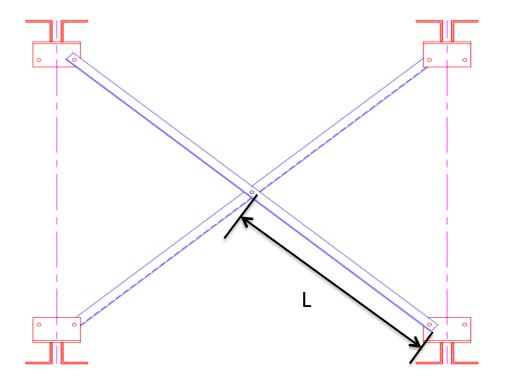
<sup>\*</sup>REFER TO LAST DIGIT(S) OF JOIST DESIGNATION

NOTE: WASHERS SHALL BE USED WITH SLOTTED OR OVERSIZED HOLES. BOLTS SHALL BE TIGHTENED TO A MINIMUM SNUG TIGHT CONDITION.



# **Bridging Connections**

Diagonal bridging must be connected at the intersection.

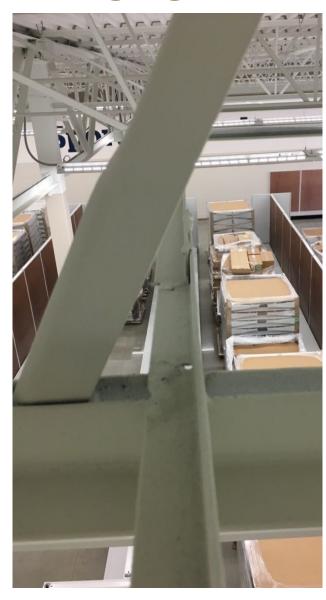


 Diagonal bridging must resist compressive axial loads and the design presumes a connection at the center of the "X", so the unbraced length is taken as the distance from the chord attachment to the center intersection.

 The center connection can be made either by welding or bolting.



## **Bridging Connections**





While the bridging weld requirements are often minimal, the material is thin and care must be exercised in making the welds.

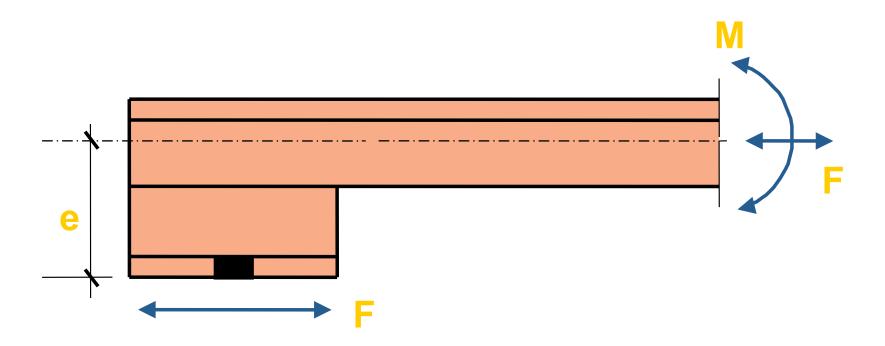


#### Simple Joist Connection Topics

- ☑ Joist to Support
- Joist Girder to Support
- **☑** Bridging Connections
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- ☐ Trade Elements Connected to Joists

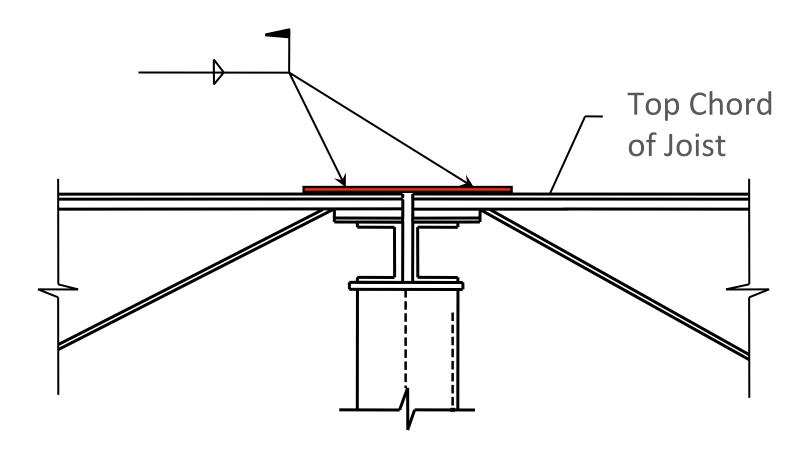


All top chord axial loads and end moments should be transmitted directly via tie plates or tie angles. The eccentricity of horizontal forces transferred through the bearing seats is then avoided.



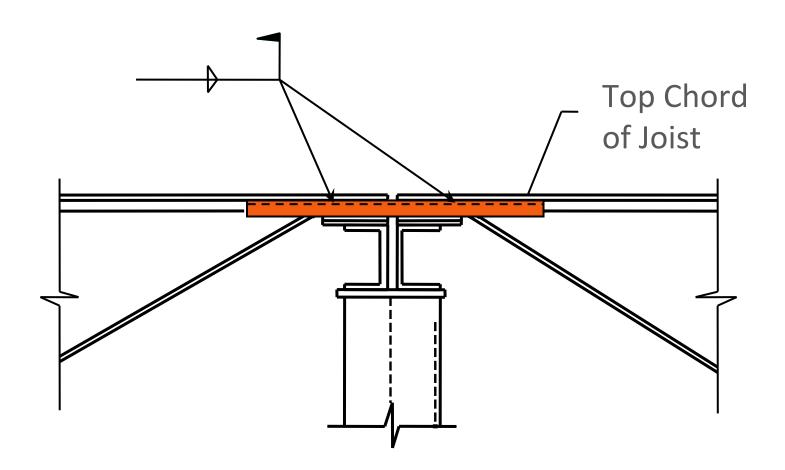


A tie plate creates an effective axial load path, without significant disruption for the deck above.





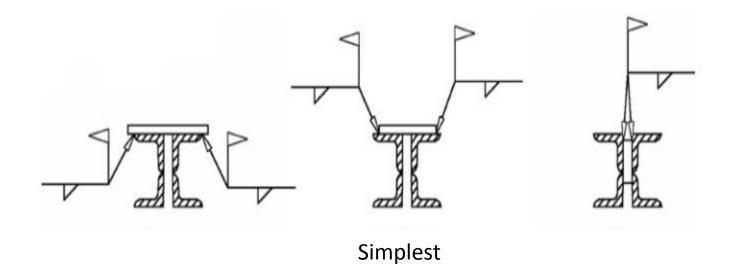
Tie angles can be tucked below the top chord, with the horizontal legs pointed in towards the joist axis.





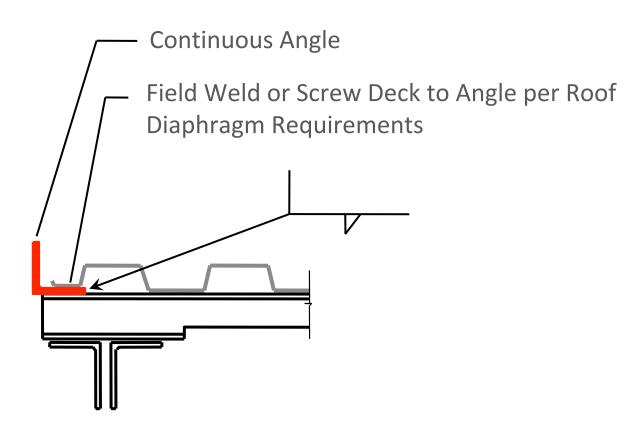
Tie plates should be narrow enough to allow downward, rather than overhead, welds.

For greater capacity, the nominal 1" chord/seat gap can be held clear for a plate to be placed in the gap.





Weld of diaphragm edge angle to top chord should not control top chord size.





#### Fillet weld practical limits:

#### At toes:

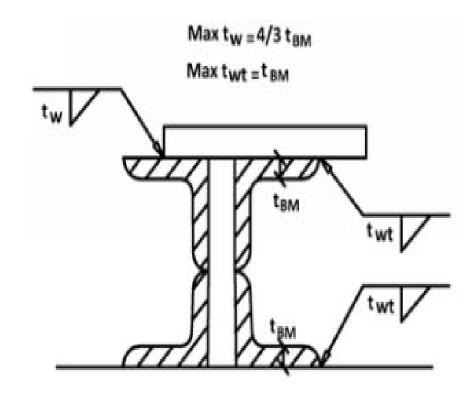
K-Series: 1/8"

LH-Series: 3/16"

Girders: 1/4"

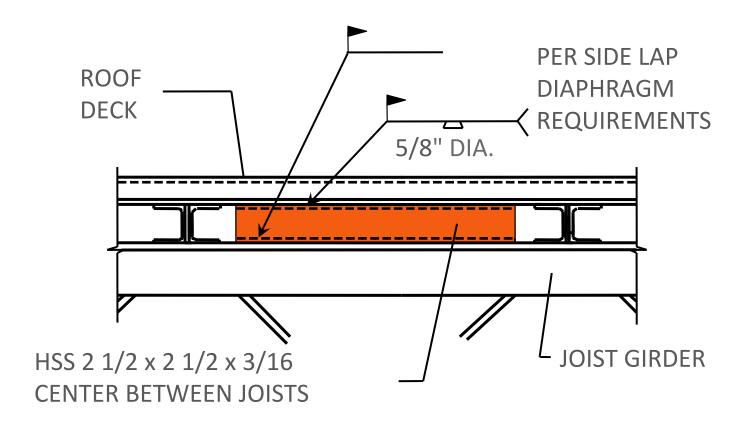
#### At legs:

4/3 of "toe" limits



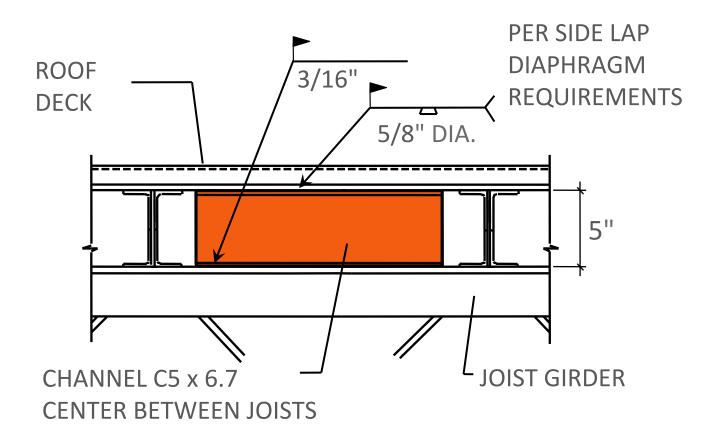


To transfer large shear forces from deck to structural members, and to avoid large joist seat rollover forces, a simple shear collector can be used.



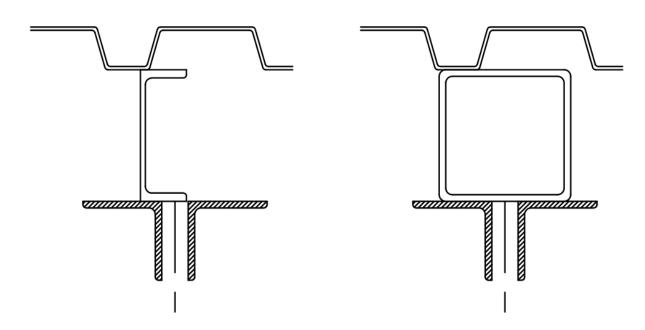


Here is a similar detail, for use with LH/DLH-Series joists.





- But maybe C5 x 6.7 is not the best choice.
- To make it simpler for the shear collector to coincide with a low deck flute, 5" HSS might be a better option.





# Simple Joist Connections



 Once the joists, Joist Girders, and bridging are installed, the connection work may not be complete!

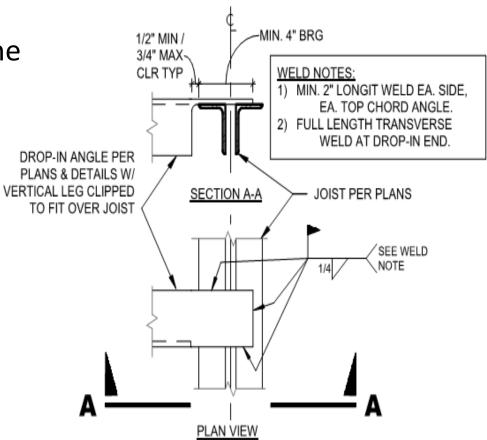


# Simple Joist Connection Topics

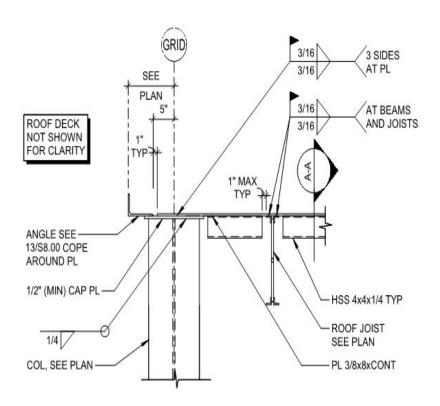
- Joist to Support
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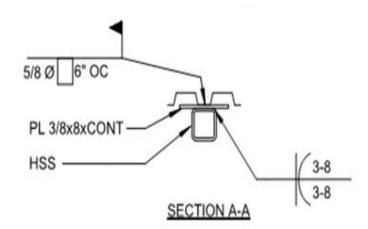
For structural members perpendicular to joists, such as an angle collector element, simply cope the angle and bear on top of the top chord.





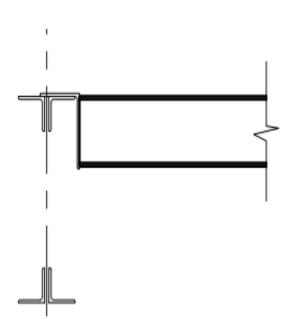


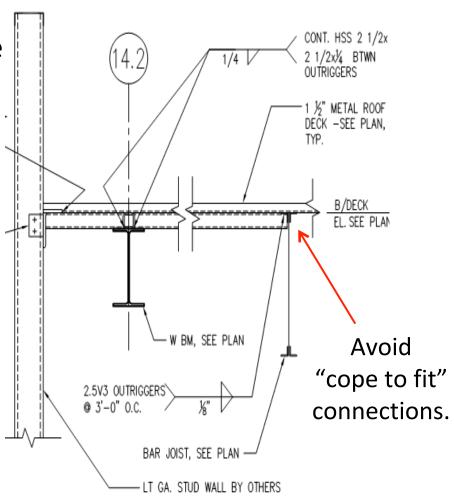
This is a similar approach with HSS and a top plate.





The next option is a hanger angle butted to the end of the structural element.





For larger vertical WF beam connections to joists, a shear tab plate can be provided on the joist chord.

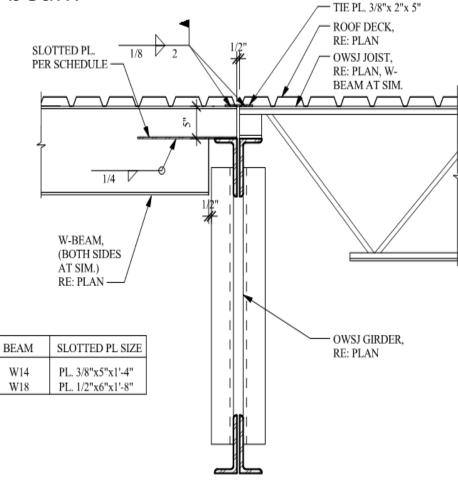


# SJE SJE

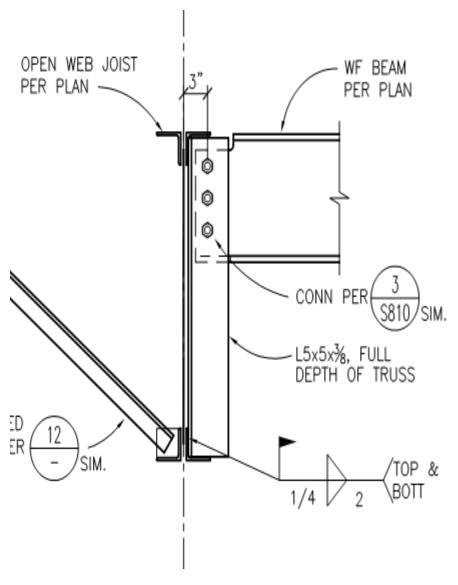
# Structural Elements Connected to Joists

Where a WF beam frames to a Joist Girder, a coped, seated beam

end is simplest.







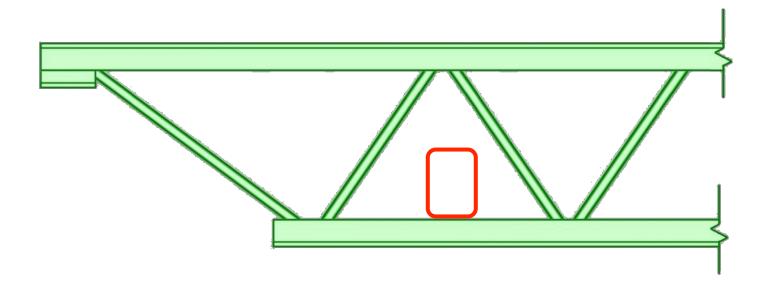
For more significant beam end reactions, a full depth connection plate or angle can be provided – shop or field installed.

This is an example of a beam connection at mid height on a deep longspan joist.

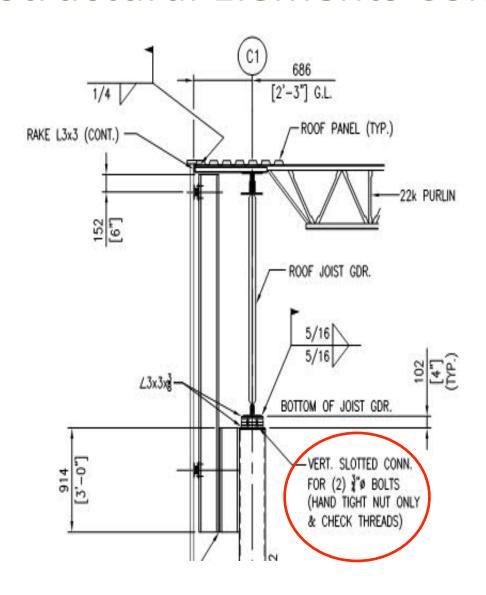




While bearing a structural element across the top of the bottom chord seems like a simple connection, it is not simple to coordinate the joist geometry to avoid interference.

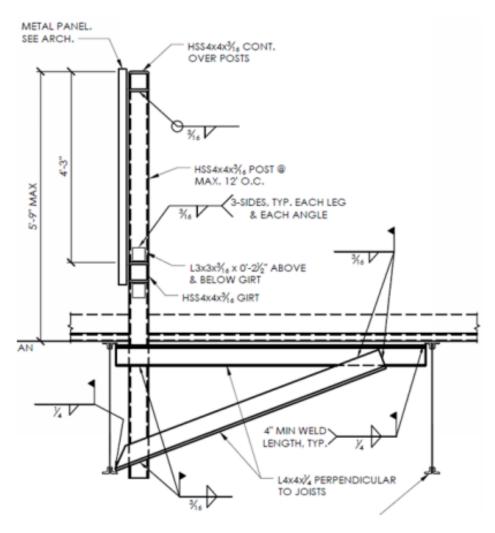






Wind column connections at midspan of a bottom chord require a vertically slotted connection.



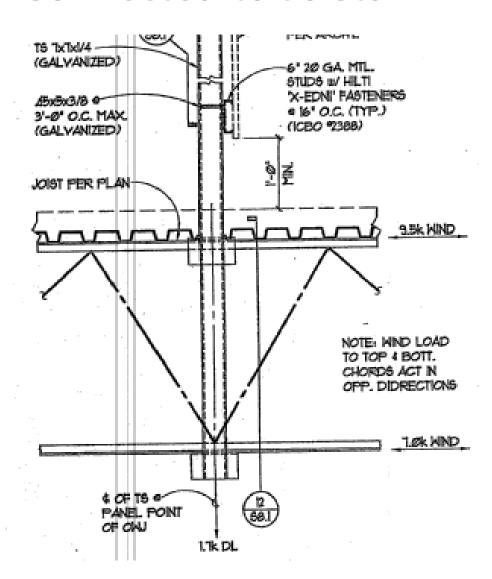


Joists simply cannot take out-of-plane torsional loads.

For a wind screen, it is best to extend the post and attach to both the top and bottom chord, resolving the overturning moment with bracing members below the roof.



Where a wind screen post is not braced, and the screen is perpendicular to the joists, extending the post and attaching to both the top and bottom chord is simpler than a large local top chord overturning moment.



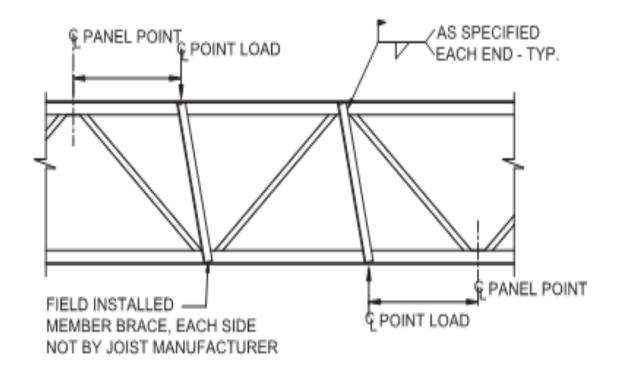


# Simple Joist Connection Topics

- ☑ Joist to Support
- ☑ Joist Girder to Support
- **T** Bridging Connections
- ✓ Lateral Load Connections
- Structural Elements Connected to Joists
- ☐ Trade Elements Connected to Joists



Added struts for point loads not located at panel points.



TYPICAL JOIST REINFORCMENT
AT CONCENTRATED LOADS



SJI allows an exception for loads that meet certain conditions.

For nominal concentrated loads between panel points, which have been accounted for in the specified uniform design loads, a "strut" to transfer the load to a panel point on the opposite chord shall not be required, provided the sum of the concentrated loads within a chord panel does not exceed 100 pounds and the attachments are concentric to the chord.



Additional capacity for trade elements can be specified, as Add-Loads, Bend-Check Loads, or a combination of both.

**Add-Load.** A single vertical concentrated load that occurs at any one panel point along the joist chord. This load is in addition to any other gravity loads specified.

**Bend-Check Load.** A vertical concentrated load used to design the joist chord for the additional bending stresses resulting from this load being applied at any location between the joist panel points. This load shall already be accounted for in the specified joist designation load, uniform load, or Add-Load and is used only for the additional bending check in the chord and does not contribute to the overall axial forces within the joist. An ideal use of this is for incidental loads which have already been accounted for in the design loading but may induce additional bending stress due to this load occurring at any location along the chord.

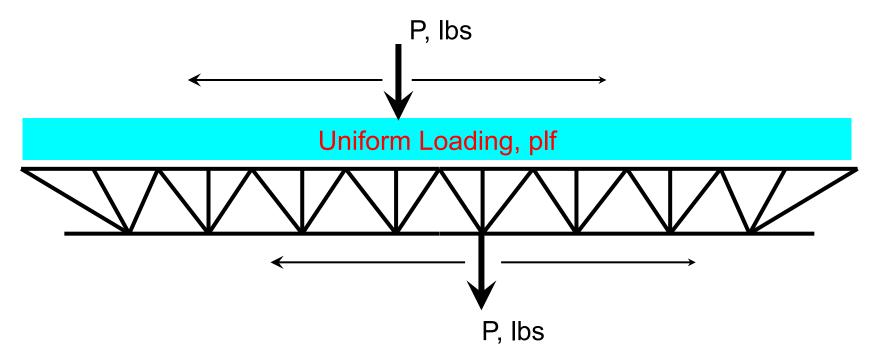


Add-Load - Traveling Loads at Panel Points

Magnitude of Concentrated Load, lbs

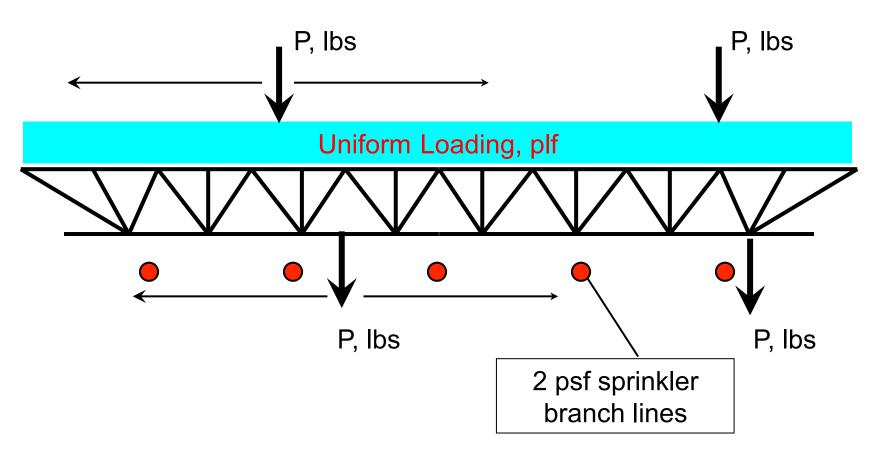
Top Chord, Bottom Chord, Either

Off Panel Point Loads Must Have Field Installed Webs



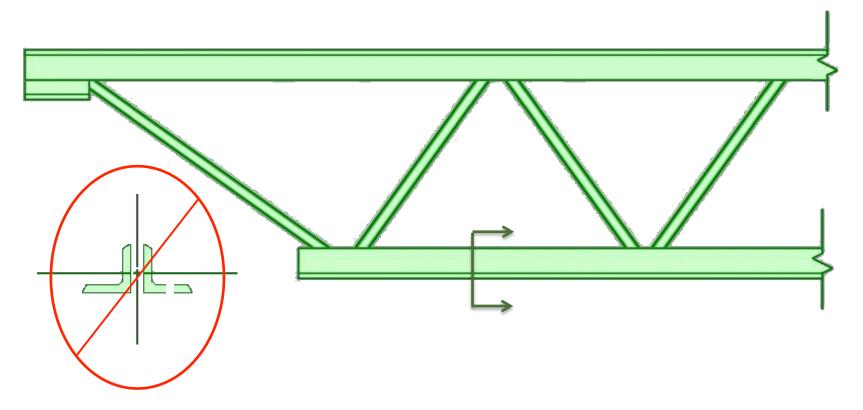


A Bend-Check Load will check the localized bending between panel points, while not adding to the global moment and shear.





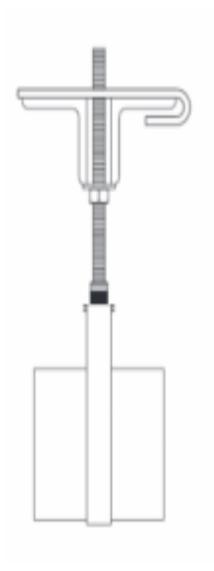
Do not field drill holes in joist members to attach trade elements.





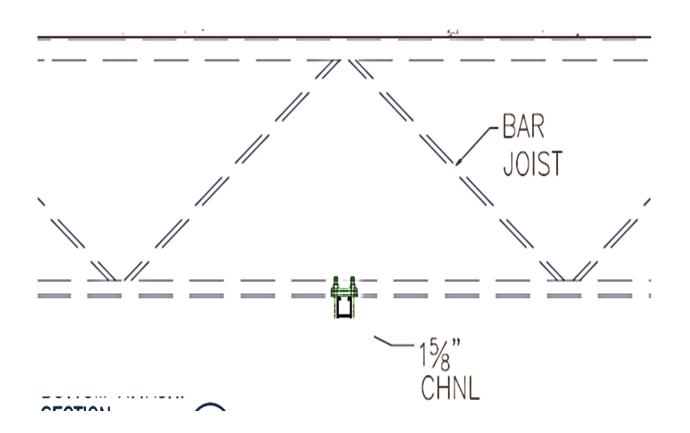
This is a simple, concentric hanger.





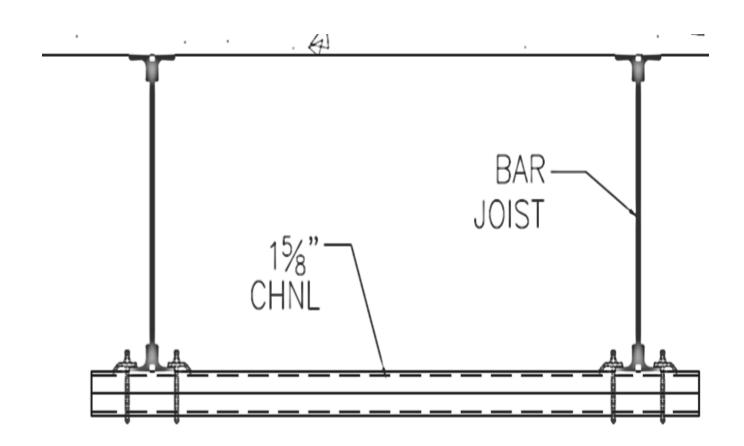


Hanger auxiliary steel example, across bottom of bottom chords.



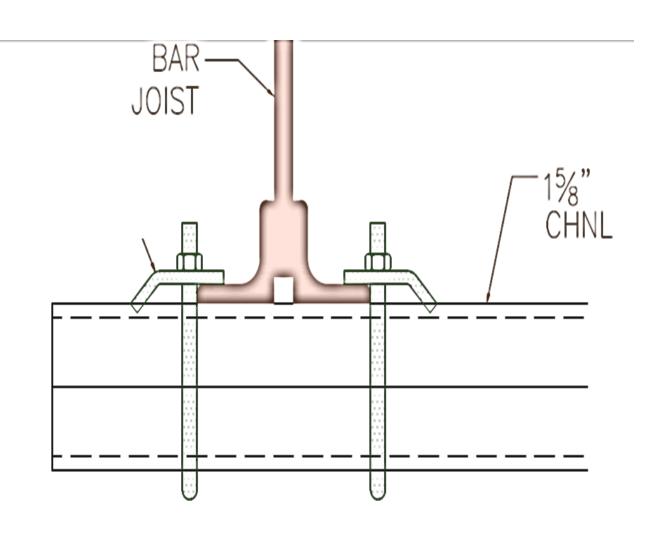


Cross section view.



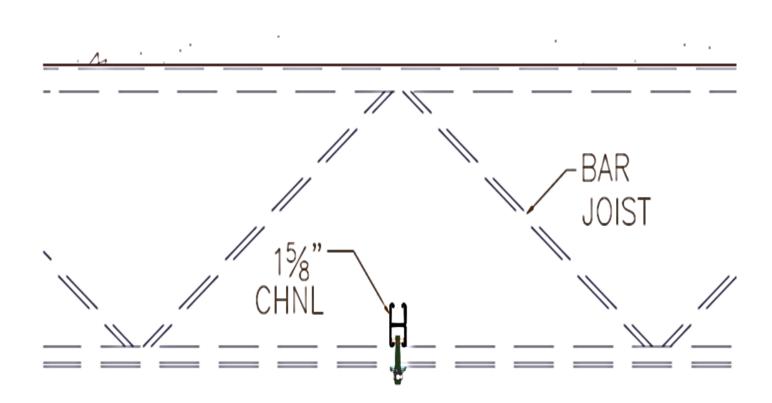


Attachment detail.



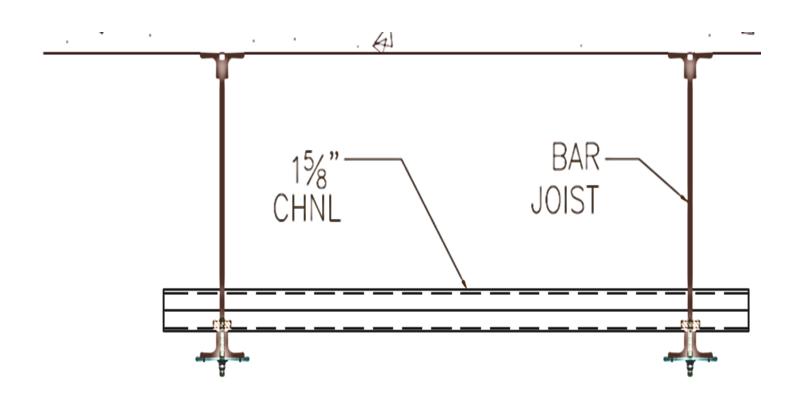


Hanger auxiliary steel across top of bottom chord chords (okay when field located).



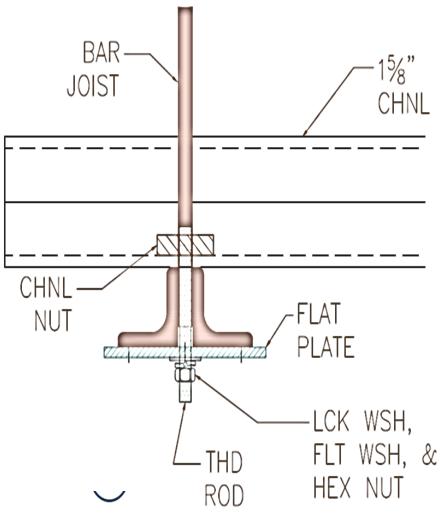


Cross section view.



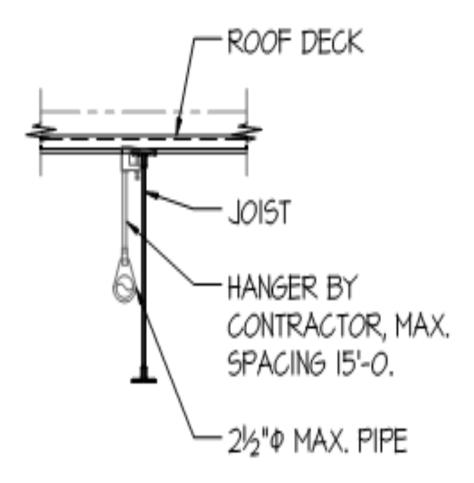


#### Attachment detail.



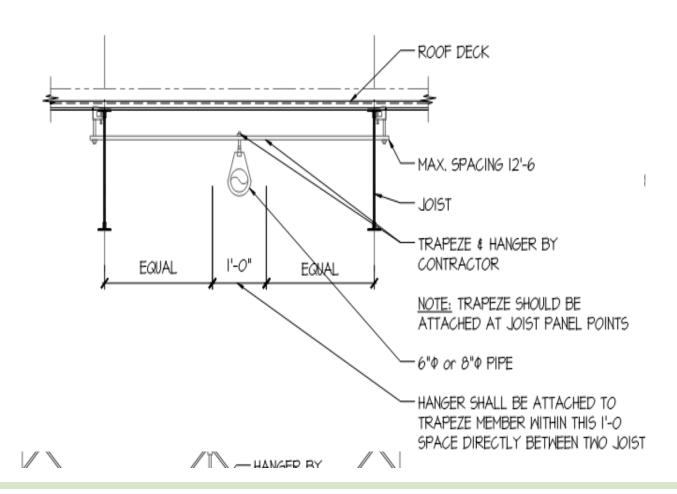


A beam clamp hanger is not concentric, and must be used with discretion, only for small loads.



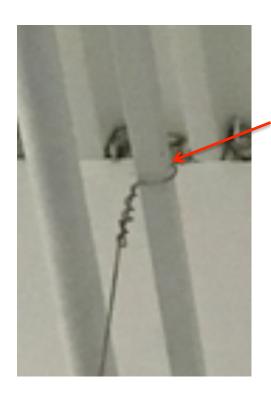


This trapeze detail limits the torsional effects of the beam clamps, but the attachment is still to only one chord angle and discretion is required.





- Wrapped wire for a light weight
- miscellaneous hanger good!
- Hung from bridging bad!







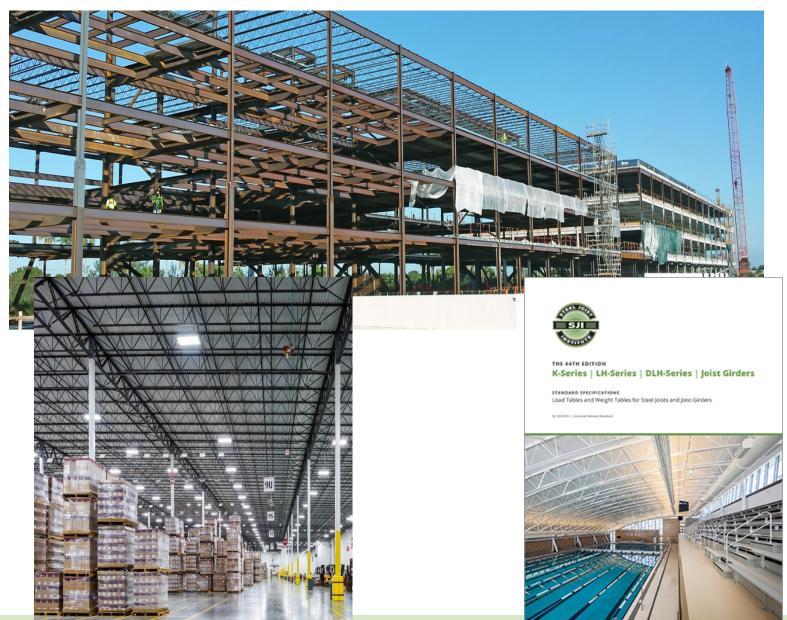
# Polling Question 1

Welded connections to/from steel joists are simplest if:

- a) The weld is as thick as possible, to minimize length.
- b) The weld is positioned such that it is done overhead.
- c) The weld is lengthened as required, to limit thickness.
- d) A multi-pass weld can be utilized.



# Simple Joist Connections Simplified





# Simple Deck Connections Simplified



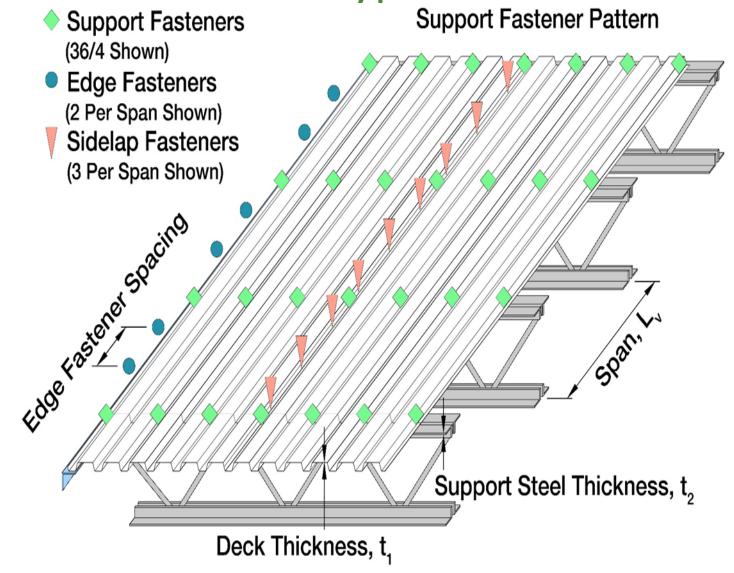


# Simple Deck Connection Topics

- ☐ Connection Types
- ☐ Connection Details
- ☐ Connection Patterns
- ☐ Connection Applications
- ☐ Hanging Load Connections
- ☐ Design Resources



# **Deck Connection Types**





# **Support Connection Choices**

#### **Welds**

#### PAF's

#### Self-Drilling Screws





#### **Relative:**

Low → High

Low → High

Low → High

Strength

••••

••000

Installation Speed

●0000

•••00

**Installed Cost** 

•••00

●0000

**Aesthetics** 

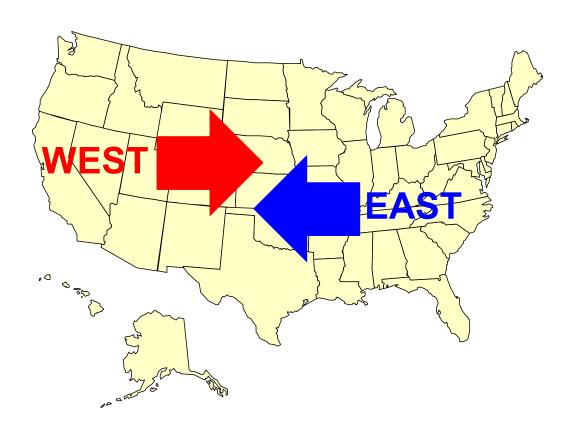
**•**0000

 $\bullet \bullet \bullet \bullet \circ$ 

•••00



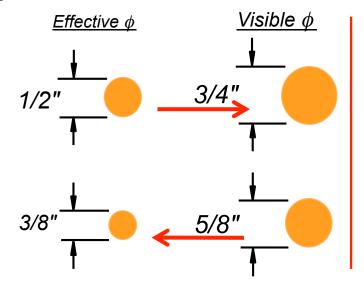
# Simply Different

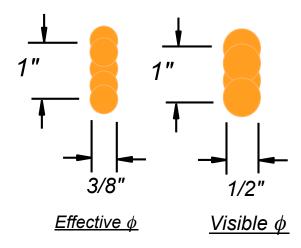




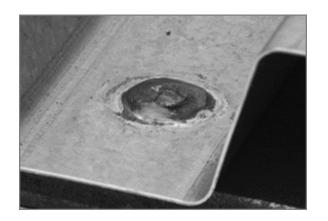
#### **Support Weld Sizes**

Arc-Spot (Puddle) Welds





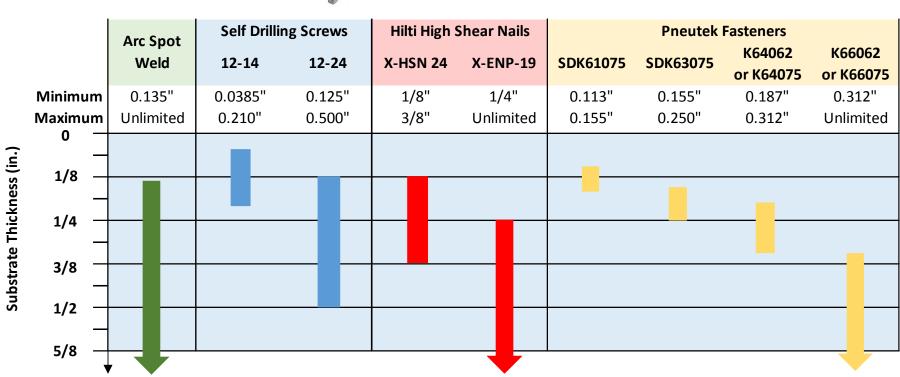
Arc Seam Welds





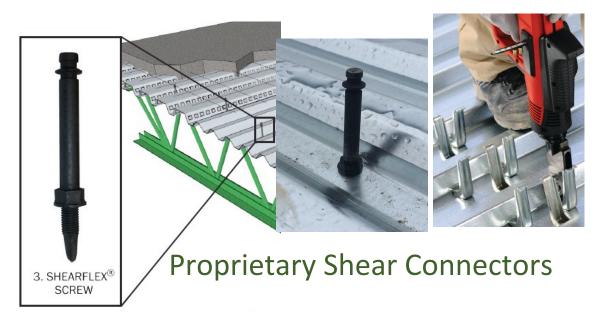
# **Support Connection Application Ranges**





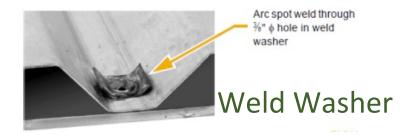


#### Concrete Filled Deck Connections



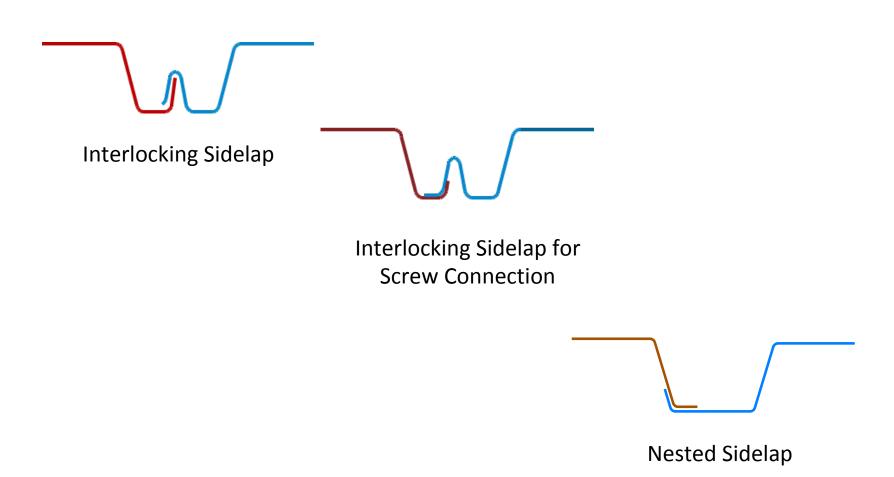
Steel Headed Stud Anchors







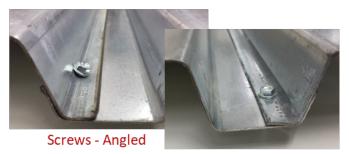
# Deck Sidelap Styles





# **Nested Sidelap Connection Choices**

<u>Screws</u> <u>Welds</u>





Screws - Vertical

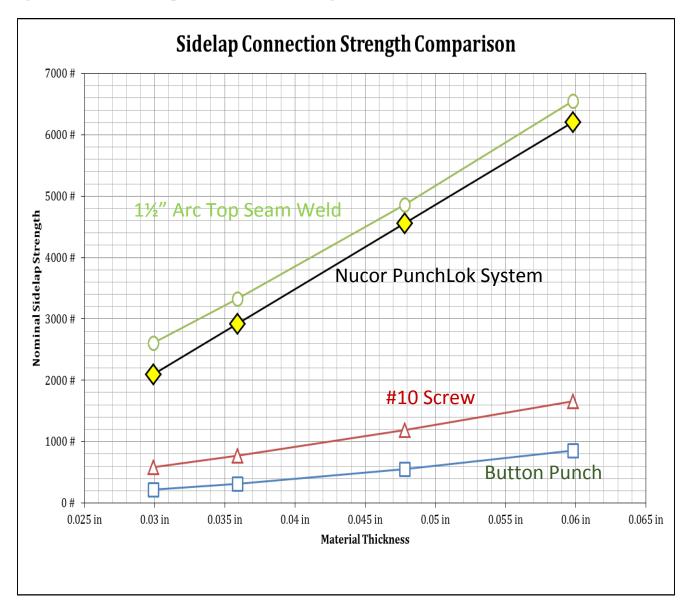
<b>Relative:</b>	Low → High	Low → High
Strength	●●○○○	••••
Installation Speed	•••00	●0000
Installed Cost	●●○○○	•••••
Aesthetics	●●○○○	●0000
Ease of Inspection		●0000

# Interlocking Sidelap Connection Choices

Clinched **Button Punches** Screws <u>Welds</u> Connections **Relative:** Low \_ High Low → High Low \_ High Low → High ••000 Strength **•**0000 **•**0000 •••00 **Installation Speed** 9000 ••000 **•**0000 **•**0000 **Installed Cost •**0000 ••000 **Aesthetics •**0000 Ease of Inspection ••000

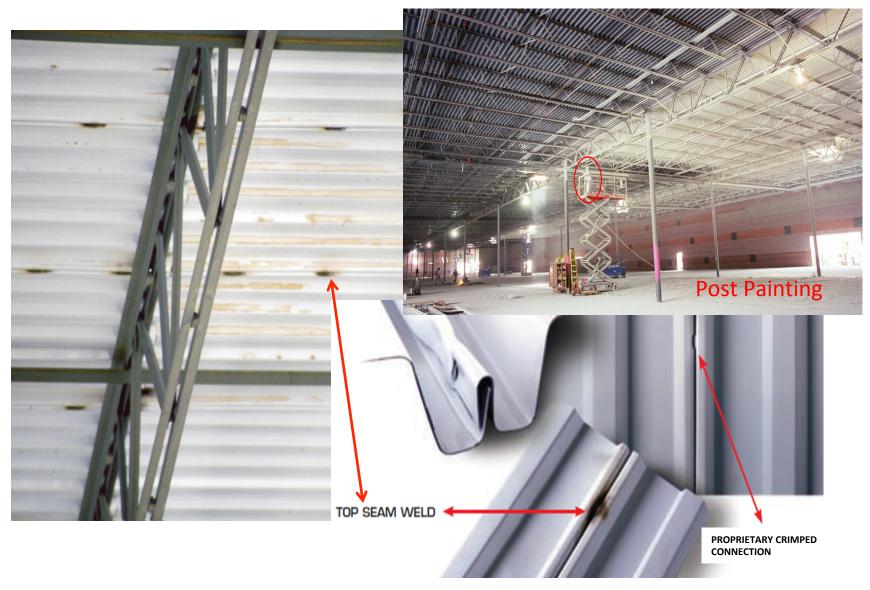


#### Sidelap Strength Comparison





#### **Deck Connection Aesthetics**



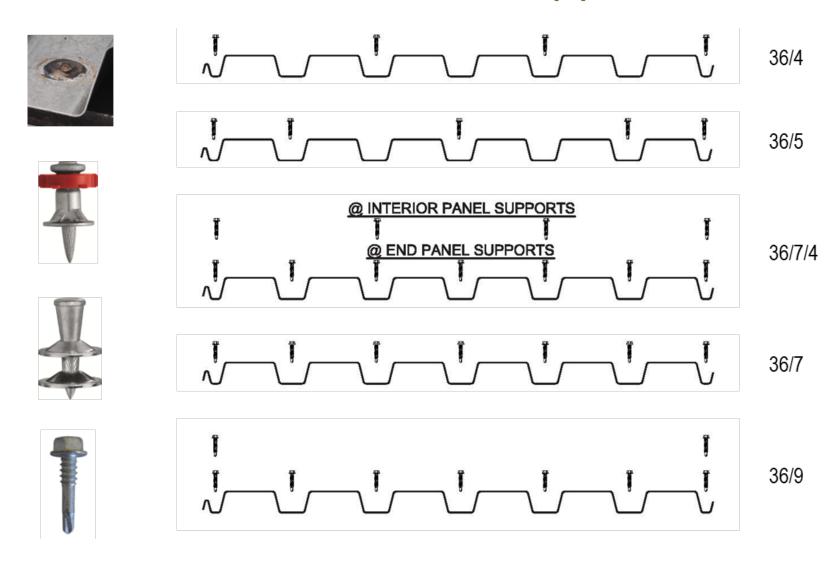


#### **Deck Connection Topics**

- ☑ Connection Types
- **☑** Connection Details
- ☐ Connection Patterns
- ☐ Connection Applications
- ☐ Hanging Load Connections
- ☐ Design Resources

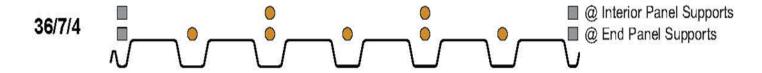


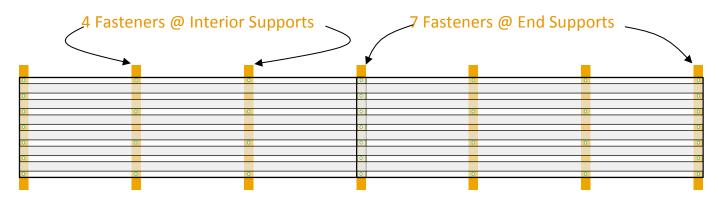
### Attachment Patterns at Supports





#### 36/7/4 Support Attachment Pattern





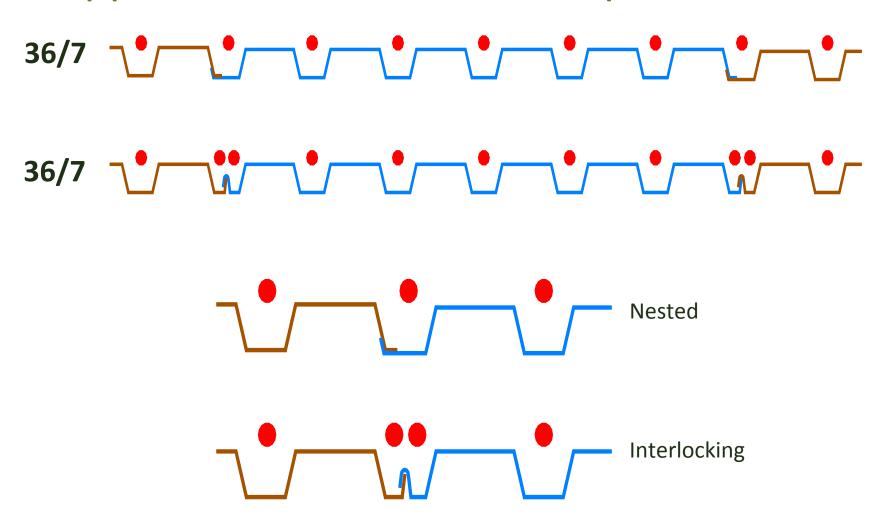
36/7/4 Pattern = 37 Fasteners over 6 Spans

#### **36/7/4 Pattern Provides:**

- Strength ≈ 36/7 Pattern, with
- ➤ Cost ≈ 36/5 Pattern.



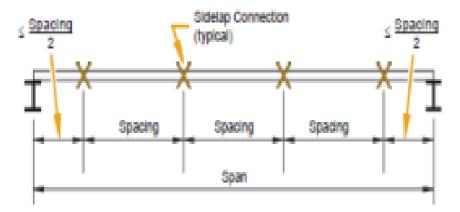
#### Support Connection at Sidelap





#### Sidelap Connection Specification

- Approaches to Presenting Tabular Data:
- # of sidelap connections / span
- Spacing of sidelap connections



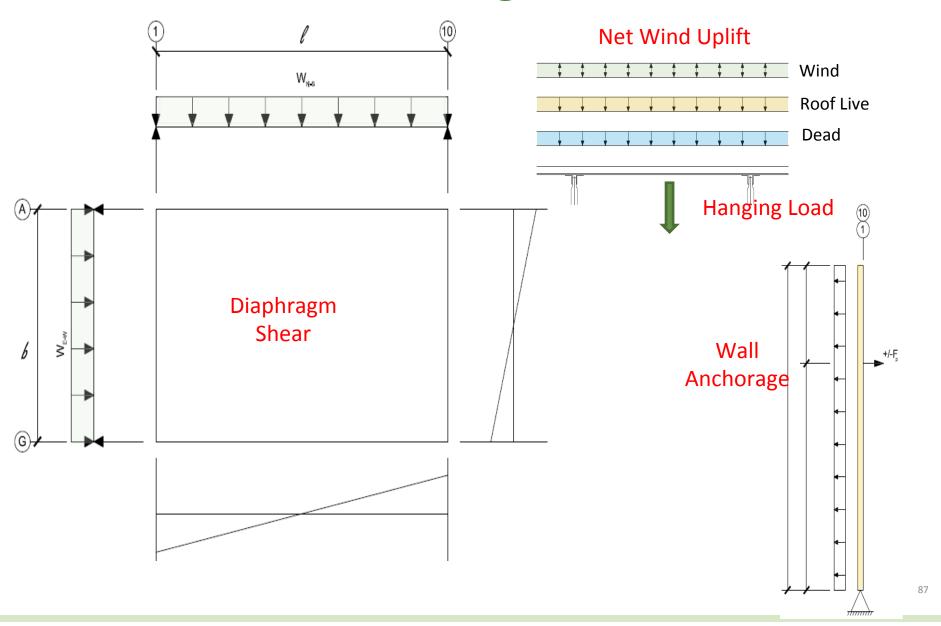
Spacing _				Span (ft)			
(in.)	4'-0"	5'-0"	6'-0"	7'-0"	8'-0"	9'-0"	10'-0"
36	2	2	2	3	3	3	4
30	2	2	3	3	4	4	4
24	2	3	3	4	4	5	5
18	3	4	4	5	6	6	7
12	4	5	6	7	8	9	10
6	8	10	12	14	16	18	20
4	12	15	18	21	24	27	30



#### **Deck Connection Topics**

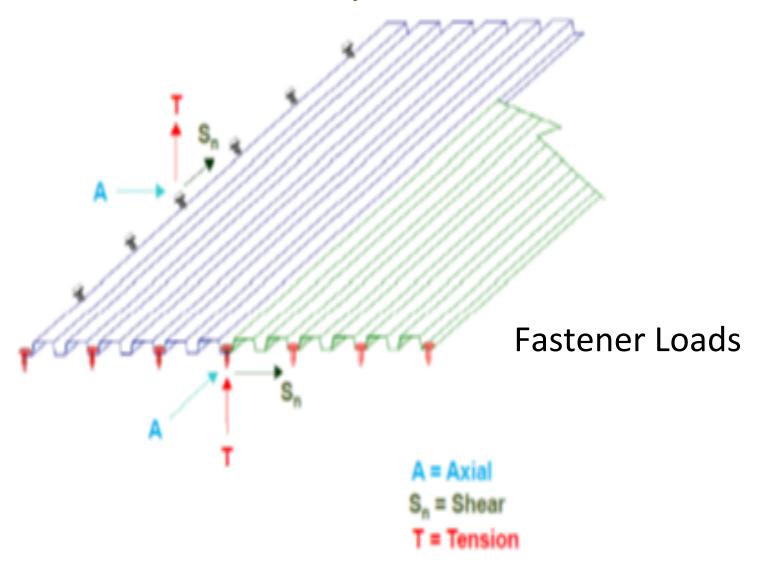
- **☑** Connection Types
- **☑** Connection Details
- **☑** Connection Patterns
- ☐ Connection Applications
- ☐ Connection of Hanging Loads
- ☐ Design Resources

### **Deck Connection Design Considerations**



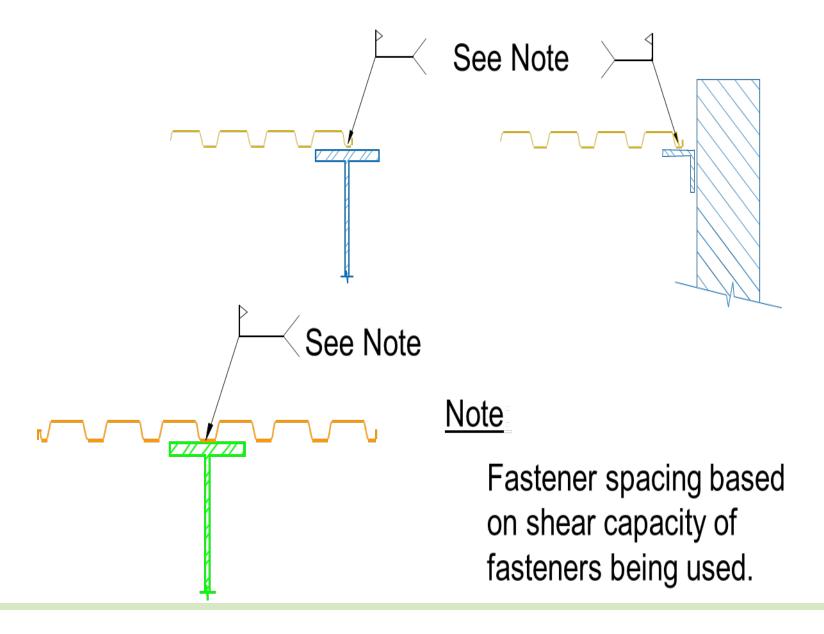


### Connection Shear / Tension Interaction



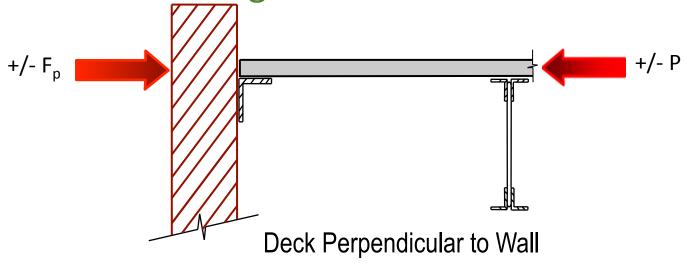


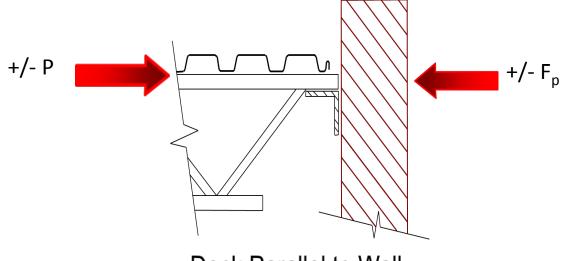
#### Parallel Collector Attachment





#### Wall Anchorage Connections

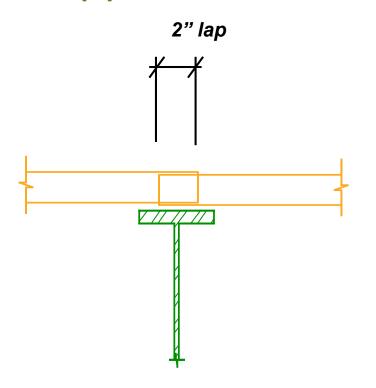




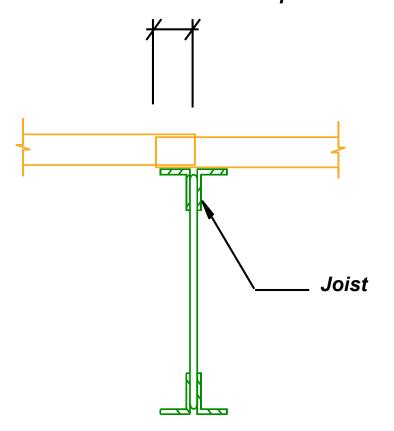
Deck Parallel to Wall



#### Support Connection Detailing – OWJ



#### Center 2" lap over chord





#### Simple Deck Connection Topics

- **☑** Connection Types
- Connection Details
- **☑** Connection Patterns
- Connection Applications
- ☐ Hanging Load Connections
- ☐ Design Resources



#### Hanging Loads – Historical Industry Guidance

per SDI Publication No. 30 (2000), Roof Deck Specification, Section 1. Scope:

"Commentary: Suspended ceilings, light fixtures, ducts, or other utilities shall not be supported by the steel deck."



# Hanging Loads – Current Industry Guidance per ANSI / SDI RD-2017

**"2.4 A.6.** Concentrated and Suspended Loads: All concentrated and suspended loads shall be included in the analysis and calculations for strength and deflection."

**"User Note:** The designer must take into account the sequence of loading. Suspended loads may include ceilings, light fixtures, ducts or other utilities. The designer should consider that loads could be applied directly to the deck rather than the support framing."

"User Note: Concentrated loads are resisted by a distribution width. A "rule of thumb" 1½ inch deck is to use the load footprint width plus 12 inches (300 mm) but not less than 18 inches (460 mm) distribution width for loads in the middle of the span."



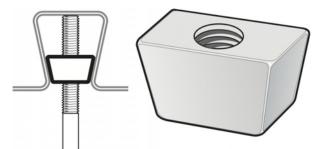
**Roof Deck Options:** 



SAMMY X-PRESS from ITW



Rolled In Hanger Tabs



Wedge Style Hangers



Self Drilling Screw Ceiling Wire Hangers

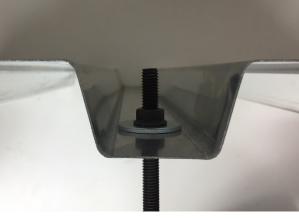


Additional Roof Deck Options:



Rebar Spreader

Fender Washer



Strut Spreader



Additional Concrete Filled Deck Options:



PAF Ceiling Wire Hangers



**Anchors** 



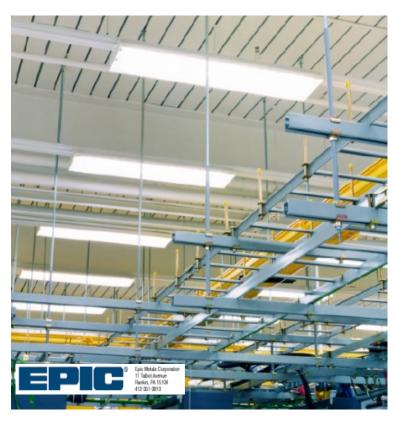
Cast-In-Place

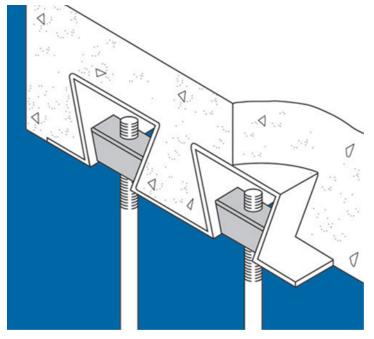






#### Additional Concrete Filled Deck Options:



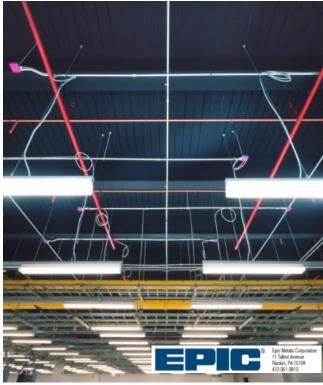


Wedge Style Hangers



Additional Concrete Filled Deck Options:







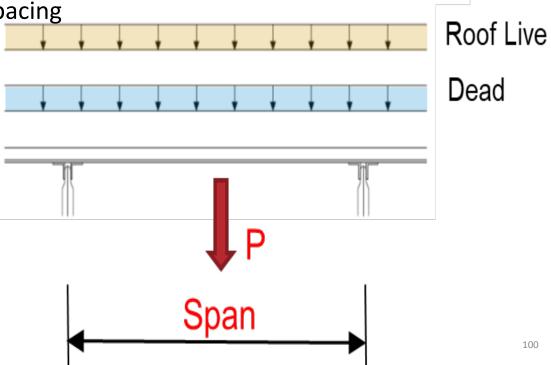
### Hanging Load Design Considerations

Determination of Allowable Hanging Loads:

- Deck gage and span
- Deck width resisting the load

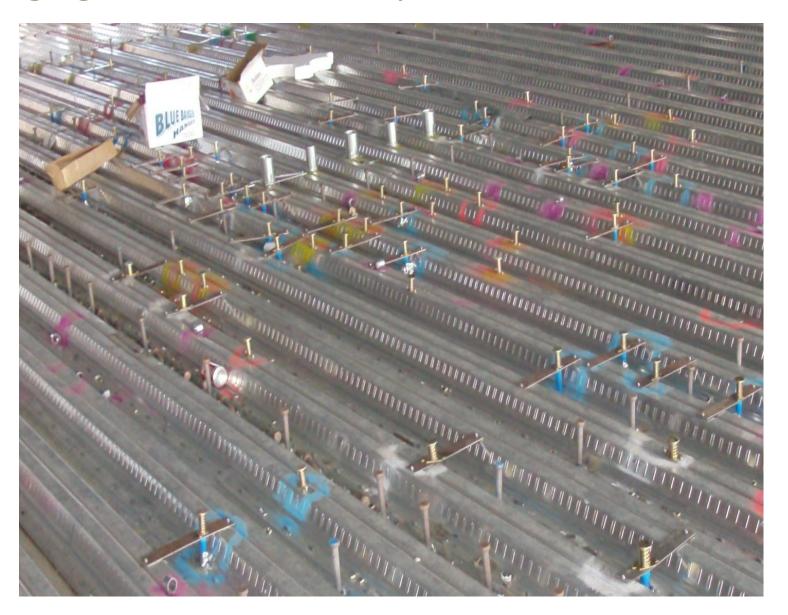
Connection capacity and spacing

Web crippling





#### Hanging Loads – Cautionary Tale





#### Deck Connection Design Resources

- SDI Literature
  - DDM
  - SDCFSFDM
  - RDDM
  - FDDM
  - On-Line Tool (Coming soon)
- Deck and Fastener Manufacturer
  - Catalogs
  - Evaluation Reports (IAPMO, ICC, etc.)
  - On-Line Tools
- AISI Standards
  - S100
  - S310





# Simple Deck Connections Simplified





### Polling Question #2

Which deck connection types are appropriate for attaching to open web joist?



d) All of the above



#### **Polling Question Answers**

Welded connections to/from steel joists are simplest if:

c) The weld is lengthened as required, to limit thickness.

Which deck connection types are appropriate for attaching to open web joist?

d) All of the above. Based on chord thickness, any one of the these connections could be appropriate, but not all choices work for every situation.



Presented by:

Tim Holtermann – Canam Buildings Jeff Martin – Vulcraft-Verco